Prime Consulting Engineers Pty. Ltd.

Design Report:

8m Dome Structure (enclosed)

For

EXTREME MARQUEES

Ref: R-22-211-2

Date: 10/05/2022

Amendment: -

Prepared by: KZ

Checked by: SD

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1 Introduction and Scope:

The report and certification are the sole property of Prime Consulting Engineers Pty. Ltd.

Prime Consulting Engineers have been engaged by Extreme Marquees Pty. Ltd. to carry out a structural analysis of 8m dome structure for wind loads region A (non-cyclonic) and snow loads for sub-alpine regions. It should be noted that the outcome of our analysis is limited to the selected items as outlined in this report.

This report shall be read in conjunction with the documents listed in the references (Section 1.2)

1.1 Project Description

The report examines the effect of 3s gust wind of **(refer to summary)** and snow loads of sub-alpine region positioned for the worst effect on the 8m dome structure. The result of this report is also applicable to the smaller dome structure with identical member sizes. The relevant Australian Standards AS1170.0:2002 General principles, AS1170.1:2002 Permanent, imposed and other actions, AS1170.2:2021 Wind actions and AS1170.3 Snow actions are used. The design check is in accordance with AS4100:1998 steel structures.

1.2 References

- The documents referred to in this report are as follows:
  - Report of results produced through SAP2000 V24 software & excel spreadsheets.
  - Detail drawing provided by manufacturer. Refer to appendix ‘A’.
- The basic standards used in this report are as follows:
  - AS 1170.0:2002 – Structural Design Actions (Part 0: General principles)
  - AS 1170.1:2002 – Structural Design Actions (Part 1: Permanent, imposed, and other actions)
  - AS 1170.2:2021 – Structural Design Actions (Part 2: Wind Actions)
  - AS 1170.3:2003 – Snow and ice actions.
  - AS4100:1998 – Steel Structures.
- Section Properties of Steel (Q235) Section provided by the client.
- The program(s) used for this analysis are as follows:
  - SAP2000 V24
  - Microsoft Excel

1.3 Notation

AS/NZS	Australian Standard/New Zealand Standard
FEM/FEA	Finite Element Method/Finite Element Analysis

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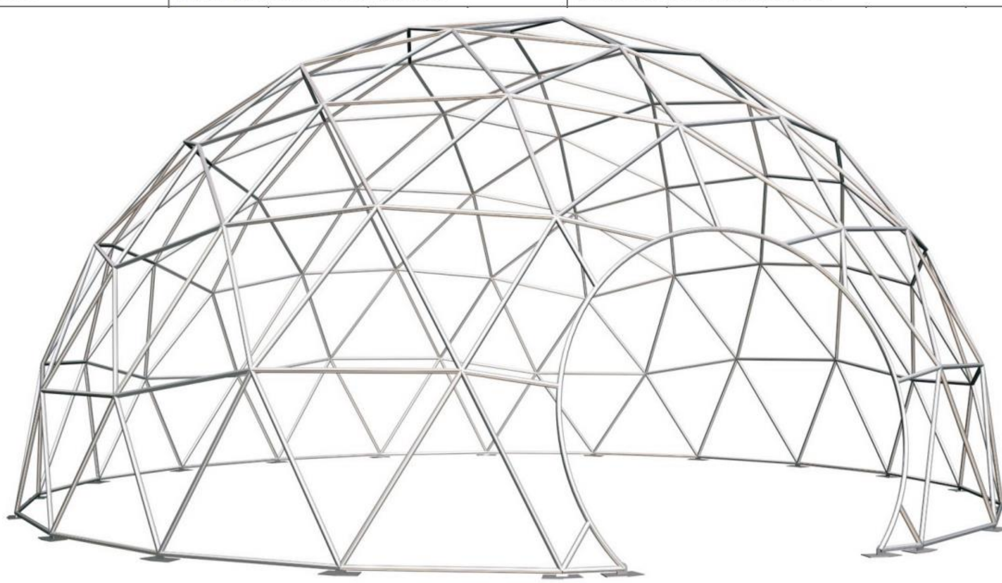
SLS	Serviceability Limit State
ULS	Ultimate Limit State

2 Design Overview

2.1 Geometry Data

ITEM	SPECIFICATION								
Size	4m	5m	6m	7m	8m	9m	10m	11m	12m
Ceiling Height	2.4m	2.5m	3m	3.5m	4m	4.5m	5m	5.5m	6m
Door Size	1200mm W x 1800mm H			1500mm W × 2100mm H			1500mm W x 2400mm H		
Dome Style	Lapela								
Floor Space	12.5 sq.m	17.8 sq.m	28 sq.m	38 sq.m	50 sq.m	63 sq.m	78 sq.m	95 sq.m	113 sq.m
Stand up Capacity	15	24	34	46	60	76	94	114	136
Sit down Capacity	10	16	23	31	40	51	63	76	90
Frame Material	Steel								
Frame Size	20mm diameter x 1.5mm thickness				25mm diameter x 2mm thickness				



Isometric view of structures

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2.2 Assumptions & Limitations

- The erected structure is for temporary use only.
- For forecast winds in excess of **(refer to summary)** the dome structure should be completely dismantled.
- The structure may only be used in regions with wind and snow classifications no greater than the limits specified in cl. 5 & 6 of this report.
- Parameters used for wind & snow calculations:
    - TC 2
    - Wind Region A
    - Sub-alpine region (Orange, NSW)
- Topographical factors such as erecting the structure on the crest of a hill or on the top of an escarpment may result in a higher wind speed classification. Thus, special considerations should be taken to the topographical location of the installation site.
- Shall the site conditions, wind or snow parameters exceed prescribed design actions (refer to cl.7), Prime Consulting Engineers Pty. Ltd. should be informed to determine appropriate wind/snow classifications and amend computations accordingly.
- It is assumed that the structure is fully enclosed with equally permeable side walls to calculate Wind Internal Forces.

2.3 Exclusions

- Design of fabric
- Wind actions due to tropical or severe tropical cyclonic areas.
- Snow actions due to snow loads other than Orange, NSW.
- Super imposed loads such as live load.

2.4 Design Parameters and Inputs

2.4.1 Load Cases

1.	G	Permanent actions (Dead load)
3.	Wu	Ultimate wind action (ULS)
4.	Ws	Serviceability wind action (SLS)
5.	S	Snow action (SLS)

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2.4.2 Load Combinations

Strength (ULS):

1.	1.35G	Permanent action only
3.	$0.9G+W_u$	Permanent and wind actions
4.	$1.2G+W_u$	Permanent and wind actions
5.	1.2G+S	Permanent and snow actions

Serviceability (SLS):

2.	$G+W_s$	Wind service actions

3 Specifications

3.1 Material Properties

Material Properties 03a - Steel Data								
Material	Fy	Fu	EffFy	EffFu	SHard	SMax	SRup	FinalSlope
Text	KN/m2	KN/m2	KN/m2	KN/m2	Unitless	Unitless	Unitless	Unitless
Q235	235000	390000	260000	430000	0.015	0.11	0.17	-0.1

3.2 Member Sizes & Section Properties

TABLE: Frame Section Properties 01 - General											
SectionName	t3	t2	tf	tw	Area	TorsConst	I33	I22	Z33	Z22	R33	R22
Text	mm	mm	mm	mm	mm2	mm4	mm4	mm4	mm3	mm3	mm	mm
20x1.5CHS	20			1.5	87.18	7508.31	3754.15	3754.15	514.5	514.5	6.562	6.562
40x40x2	40	40	2	2	304	109744	73365.33	73365.33	4336	4336	15.535	15.535

4 Design Loads

Self weight	G	self weight
3s 100km/hr gust	W_u	0.383 C_{fig} (kPa)
Sub-alpine snow load	W_s	0.33 - 0.64 (kPa)

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5 Wind Analysis

5.1 Ultimate

PRIME CONSULTING ENGINEERS PTY. LTD

Project: *8m Dome Structure*

Jon no. *22-211-2* *Designer:* *KZ*

Date: *4/05/2022* *Amendment:* *-*

Name	*Symbol*	*Value*	*Unit*	*Notes*	*Ref.*
			General		
Importance level		3			*Table 3.1 - Table 3.2 (AS1170.0)*
Annual probability of exceedance		Temporary			*Table 3.3*
Regional gust wind speed		**100.00**	Km/hr		
Regional gust wind speed	V_R	27.777	m/s		
Wind Direction Multipliers	M_d	1			*Table 3.2 (AS1170.2)*
Terrain Category	TC	2			
Terrain Category Multiplier	$M_{Z,Cat}$	0.91			
Shield Multiplier	M_S	1			*4.3 (AS1170.2)*
Topographic Multiplier	M_t	1			*4.4 (AS1170.2)*
Site Wind Speed	$V_{Site,\beta}$	**25.28**	m/s	$V_{Site,\beta}=V_R*M_d*M_{z,cat}*M_S,M_t$	
Pitch	α	0	Deg		
Pitch	α	0.000	rad		
Width	B	8	m		
Width Span	S_w	-	m		
Length	D	8	m		
Height	Z	2	m		
Bay Span		-	m		
	h/d	0.25			
	h/b	0.25			
			Wind Pressure		
ρair	ρ	1.2	Kg/m³		

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dynamic response factor	C_{dyn}	1			
Wind Pressure	ρ*C_{fig}	**0.383**	Kg/m^2	*ρ=0.5ρ_{air}*($V_{des,\beta}$)^2*C_{fig}*C_{dyn}*	*2.4 (AS1170.2)*
WIND DIRECTION 1 (Perpendicular to Length)					
Internal Pressure					
Opening Assumption					
	Without Dominant Opening				
Internal Pressure Coefficient (Without Dominant) **MIN**		-0.3			
Internal Pressure Coefficient (Without Dominant) **MAX**		0.2			
Internal Pressure Coefficient (With Dominant) **MIN**		0			
Internal Pressure Coefficient (With Dominant) **MAX**		0.3			
N		0.7		*Cpi= N*Cpe*	
Combination Factor	$K_{C,i}$	1			
Internal Pressure Coefficient **MIN**	$C_{p,i}$	-0.30			
Internal Pressure Coefficient **MAX**	$C_{p,i}$	0.20			
External Pressure					
1. Windward Wall					
External Pressure Coefficient	$C_{P,e}$	0.7			
Area Reduction Factor	K_a	1			*Table 5.4*
combination factor applied to internal pressures	$K_{C,e}$	0.8			
local pressure factor	K_l	1			
porous cladding reduction factor	K_p	1			
aerodynamic shape factor	$C_{fig,e}$	0.56			
Wind Wall Pressure	P	**0.21**	**kPa**		
2. Leeward Wall					
External Pressure Coefficient	$C_{P,e}$	-0.5			
Area Reduction Factor	K_a	1			*Table 5.4*
combination factor applied to internal pressures	$K_{C,e}$	0.8			
local pressure factor	K_l	1			
porous cladding reduction factor	K_p	1			

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aerodynamic shape factor	$C_{fig,e}$	-0.4			
Leeward Wall Pressure	P	**-0.15**	**kPa**		
3. Side Wall					
Area Reduction Factor	K_a	1			*Table 5.4*
combination factor applied to internal pressures	$K_{C,e}$	0.8			
local pressure factor	K_l	1			
porous cladding reduction factor	K_p	1			
External Pressure Coefficient	$C_{P,e}$	-0.65		*0 to 1h*	
External Pressure Coefficient	$C_{P,e}$	-0.5		*1h to 2h*	
External Pressure Coefficient	$C_{P,e}$	-0.3		*2h to 3h*	
External Pressure Coefficient	$C_{P,e}$	-0.2		*>3h*	
aerodynamic shape factor	$C_{fig,e}$	-0.52		*0 to 1h*	
aerodynamic shape factor	$C_{fig,e}$	-0.4		*1h to 2h*	
aerodynamic shape factor	$C_{fig,e}$	-0.24		*2h to 3h*	
aerodynamic shape factor	$C_{fig,e}$	-0.16		*>3h*	
Side Wall Pressure	P	**-0.20**	**kPa**	*0 to 1h*	
Side Wall Pressure	P	**-0.15**	**kPa**	*1h to 2h*	
Side Wall Pressure	P	**-0.09**	**kPa**	*2h to 3h*	
Side Wall Pressure	P	**-0.06**	**kPa**	*>3h*	
4. Roof					
r (rise)	r	4	**m**		
h/r	h/r	0.50			
Breadth Effect		1.00		*(b/d)^0.25>1*	
Rise-to-span ratio	r/d	0.50			
4.1 Roof Windward Quarter					
U	U	2	**m**		Table C3
Area Reduction Factor	K_a	1			
combination factor applied to internal pressures	$K_{C,e}$	0.8			
local pressure factor	K_l	1			
porous cladding reduction factor	K_p	1			
External Pressure Coefficient	$C_{P,e}$	0.3			
Factored External Pressure Coefficient	$C_{P,e}$	0.30			
aerodynamic shape factor	$C_{fig,e}$	0.24			
Pressure	P	**0.09**	**kPa**		

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4.2 Roof Centre Half				
T	T	4	**m**	Table C3
Area Reduction Factor	K_a	1		
combination factor applied to internal pressures	$K_{C,e}$	0.8		
local pressure factor	K_l	1		
porous cladding reduction factor	K_p	1		
External Pressure Coefficient	$C_{P,e}$	-0.65		
Factored External Pressure Coefficient	$C_{P,e}$	-0.65		
aerodynamic shape factor	$C_{fig,e}$	-0.52		
Pressure	P	**-0.20**	**kPa**	
4.2 Roof Centre Half				
D	D	2	**m**	Table C3
Area Reduction Factor	K_a	1		
combination factor applied to internal pressures	$K_{C,e}$	0.8		
local pressure factor	K_l	1		
porous cladding reduction factor	K_p	1		
External Pressure Coefficient	$C_{P,e}$	-0.2		
Factored External Pressure Coefficient	$C_{P,e}$	-0.20		
aerodynamic shape factor	$C_{fig,e}$	-0.16		
Pressure	P	**-0.06**	**kPa**	

5.1.1 Summary Forces

WIND EXTERNAL PRESSURE (kPa)				
			Wind Direction	
Windward			0.21	
Leeward			-0.15	
Sidewall	0m - 2m		-0.20	
	2m - 4m		-0.15	
	4m - 6m		-0.09	
	> 6m		-0.06	
Roof				
	Windward Quarter (U)	2m	0.09	
	Centre Half (T)	4m	-0.20	
	Leeward Quarter (D)	2m	-0.06	
Wind Internal Pressure (kPa)				
			-0.12	0.08

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6 Snow Load

6.1 Sub-Alpine

S = Sg x Ce x μi

Annual probability of exceedance: 1/20
Snow region: Orange, NSW
Sg = 0.9kPa

C_e = 1

Average slope:
Segment 1: 9° → μ= = 0.71
Segment 2: 36° → μ= = 0.37
Segment 3: 72° → μ= = 0

Snow loads:
Segment 1: 0.64 kPa
Segment 2: 0.33 kPa
Segment 3: 0.0 kPa

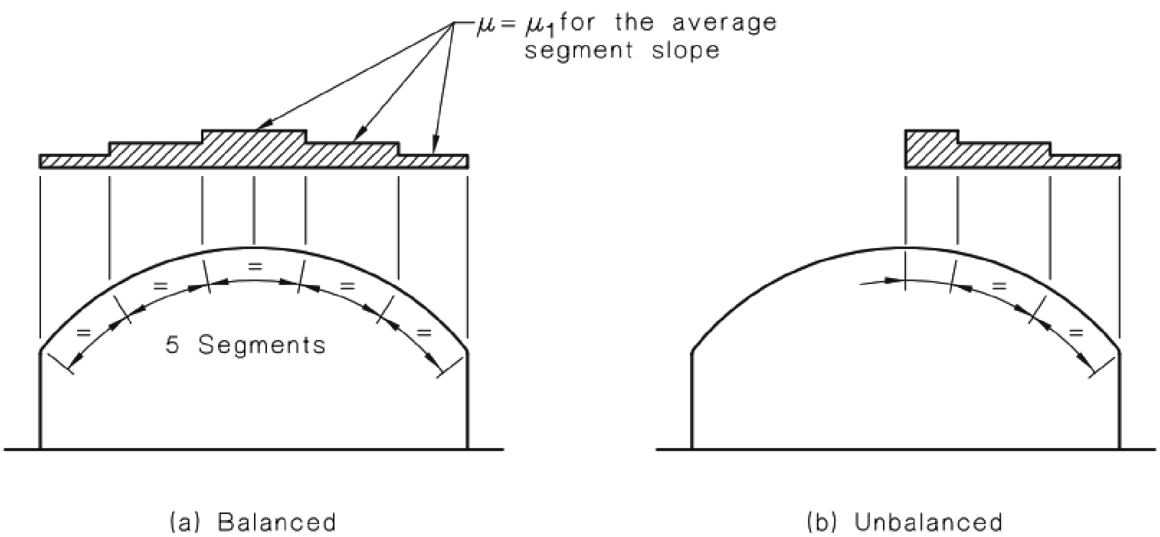

FIGURE 7.4 TYPES OF SNOW LOAD ON CURVED ROOFS (SUB-ALPINE)

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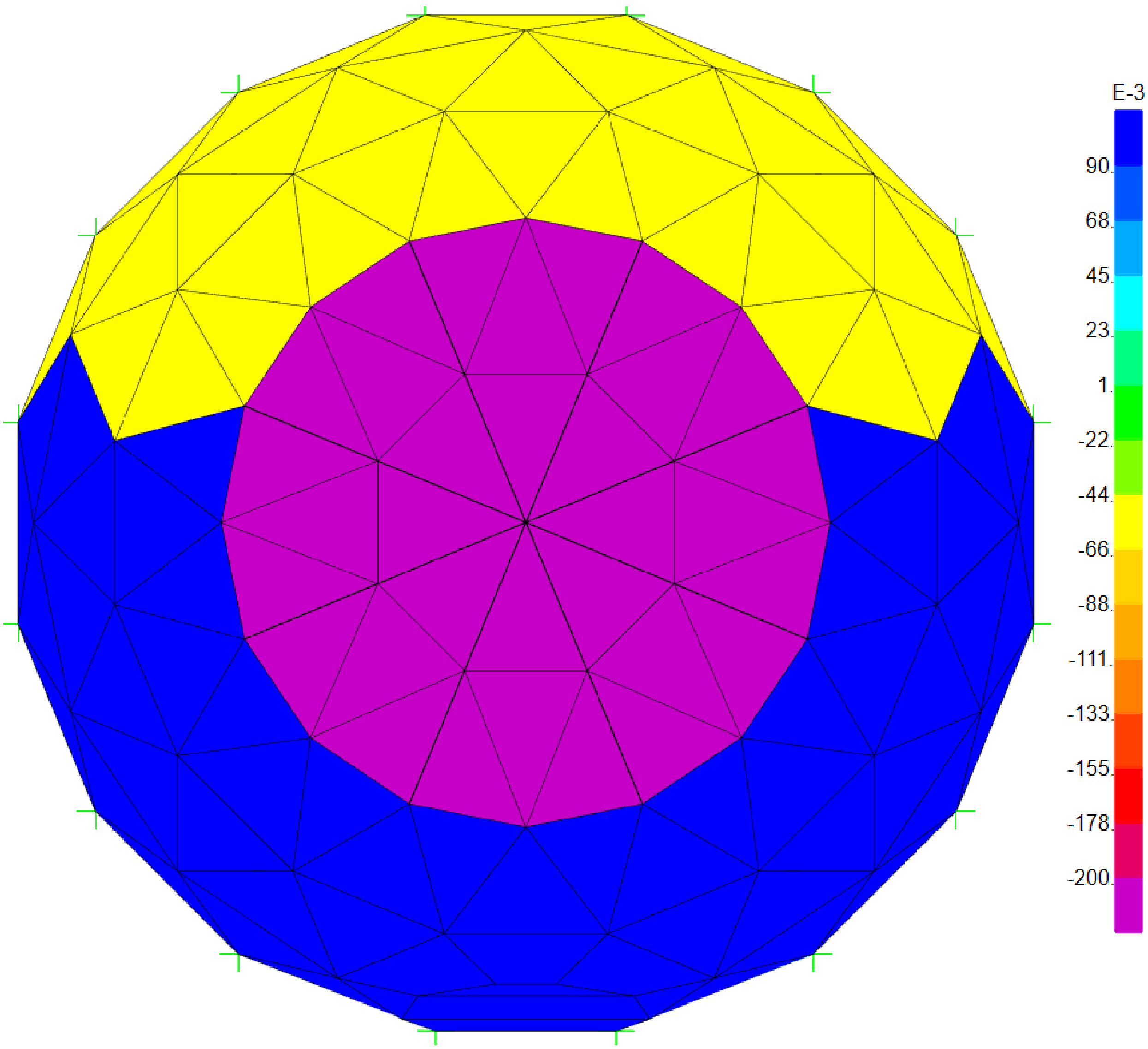
7 Load Diagrams

7.1 Wind Load

7.1.1 Wind Load Ultimate (W_U)



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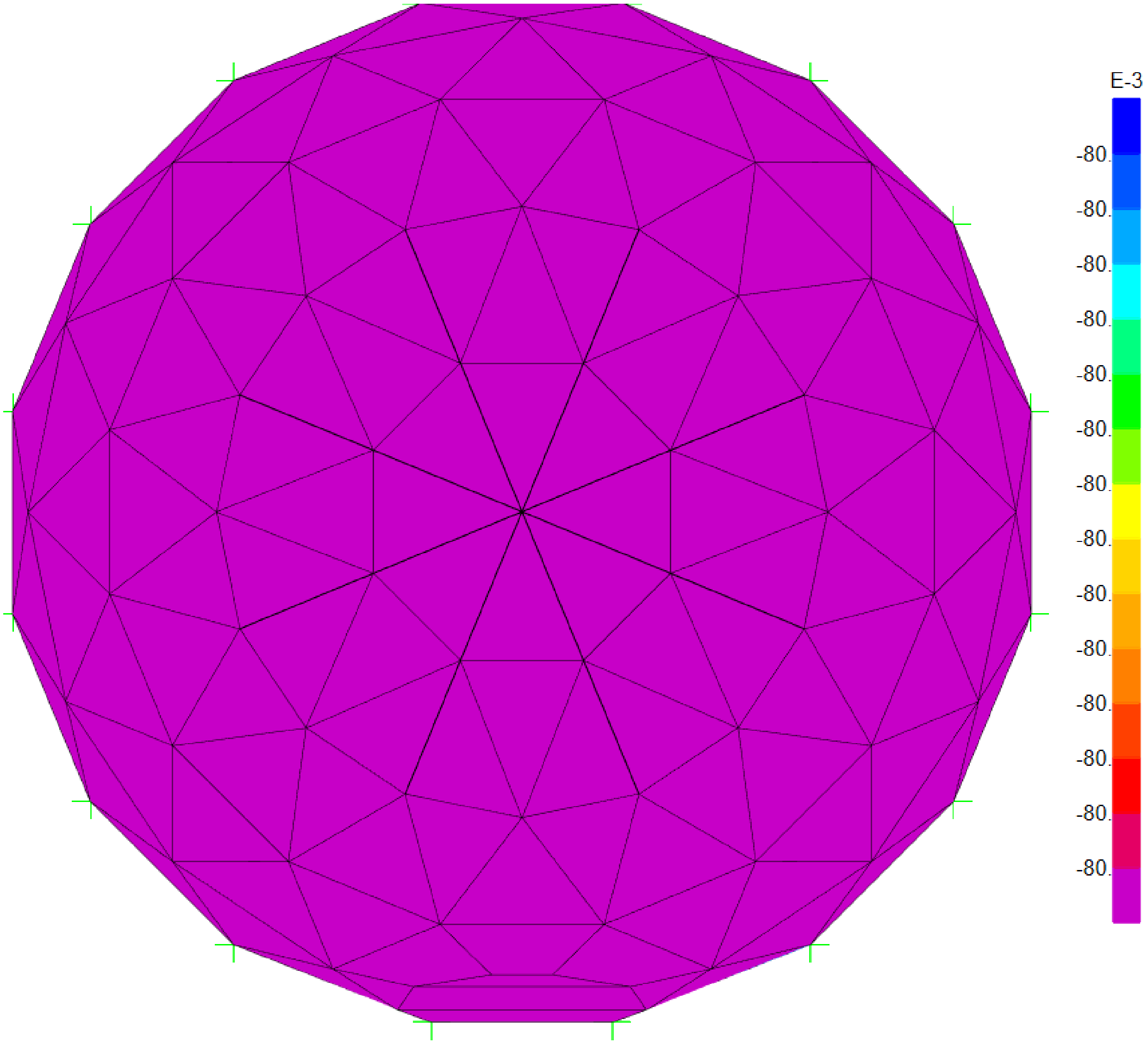
7.1.2 Wind Load Internal Pressure ($W_{I,pressure}$)



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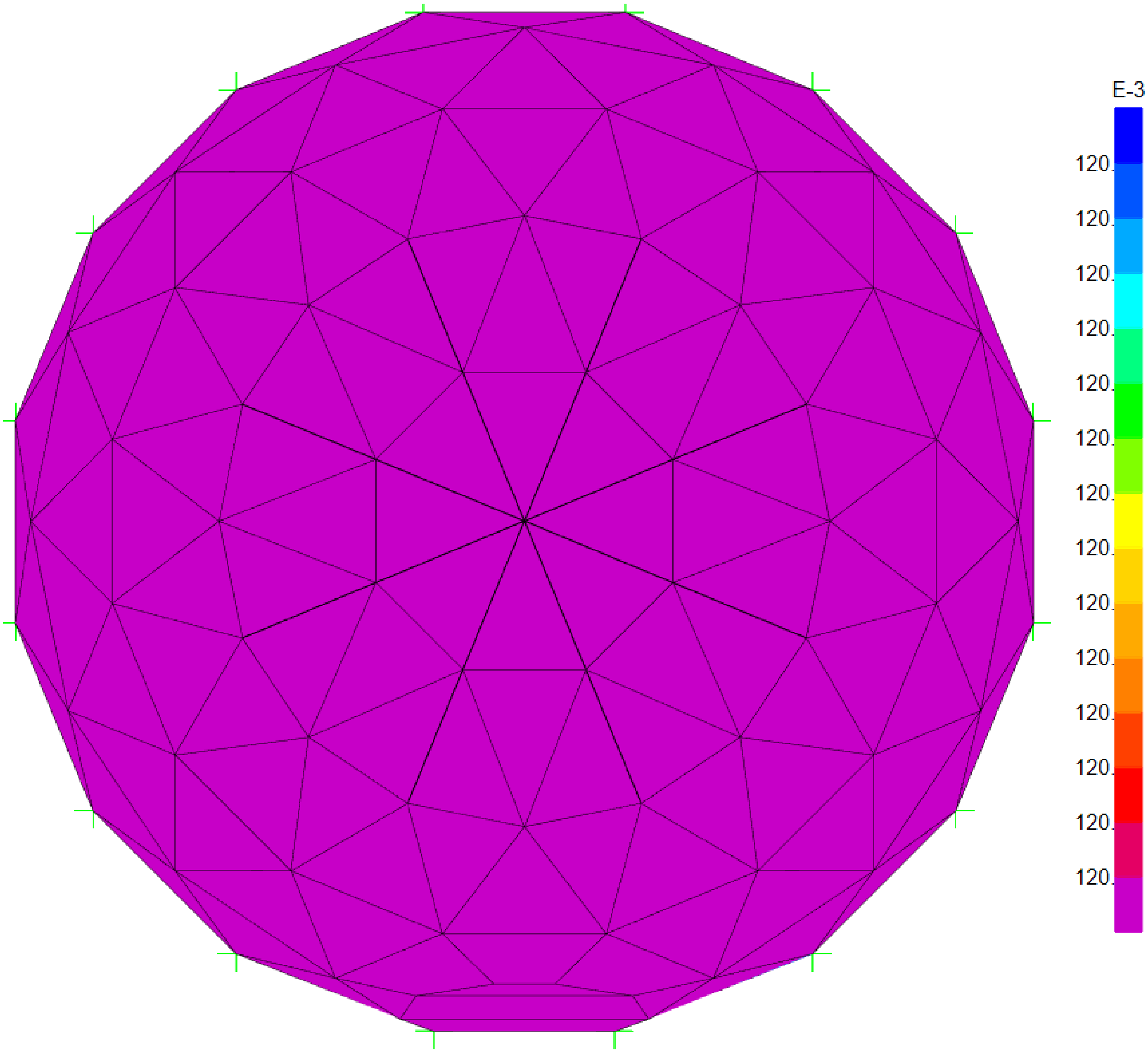
7.1.3 Wind Load Internal Suction ($W_{I,suction}$)



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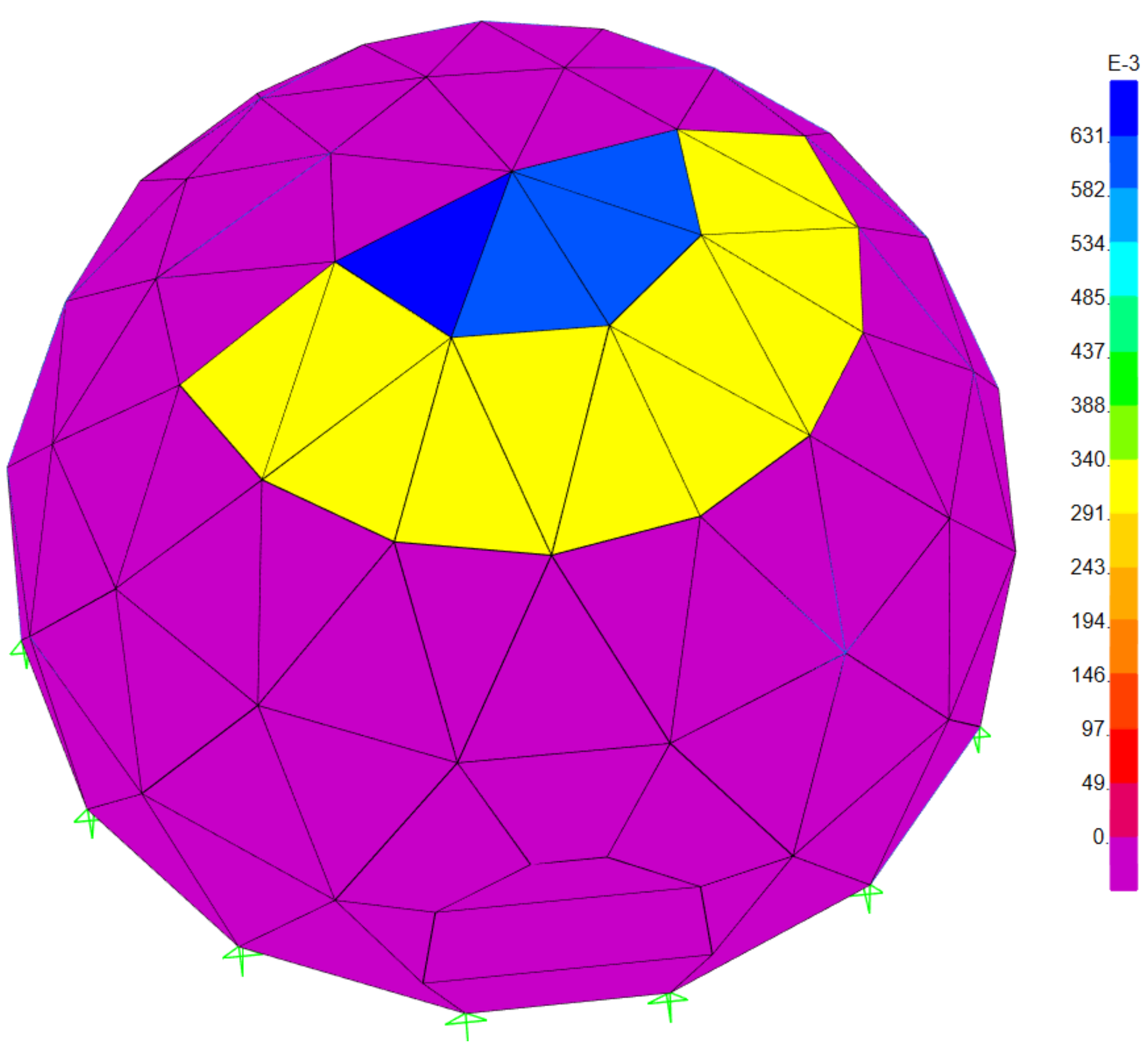
7.2 Snow Load

7.2.1 Snow Load (Case1)



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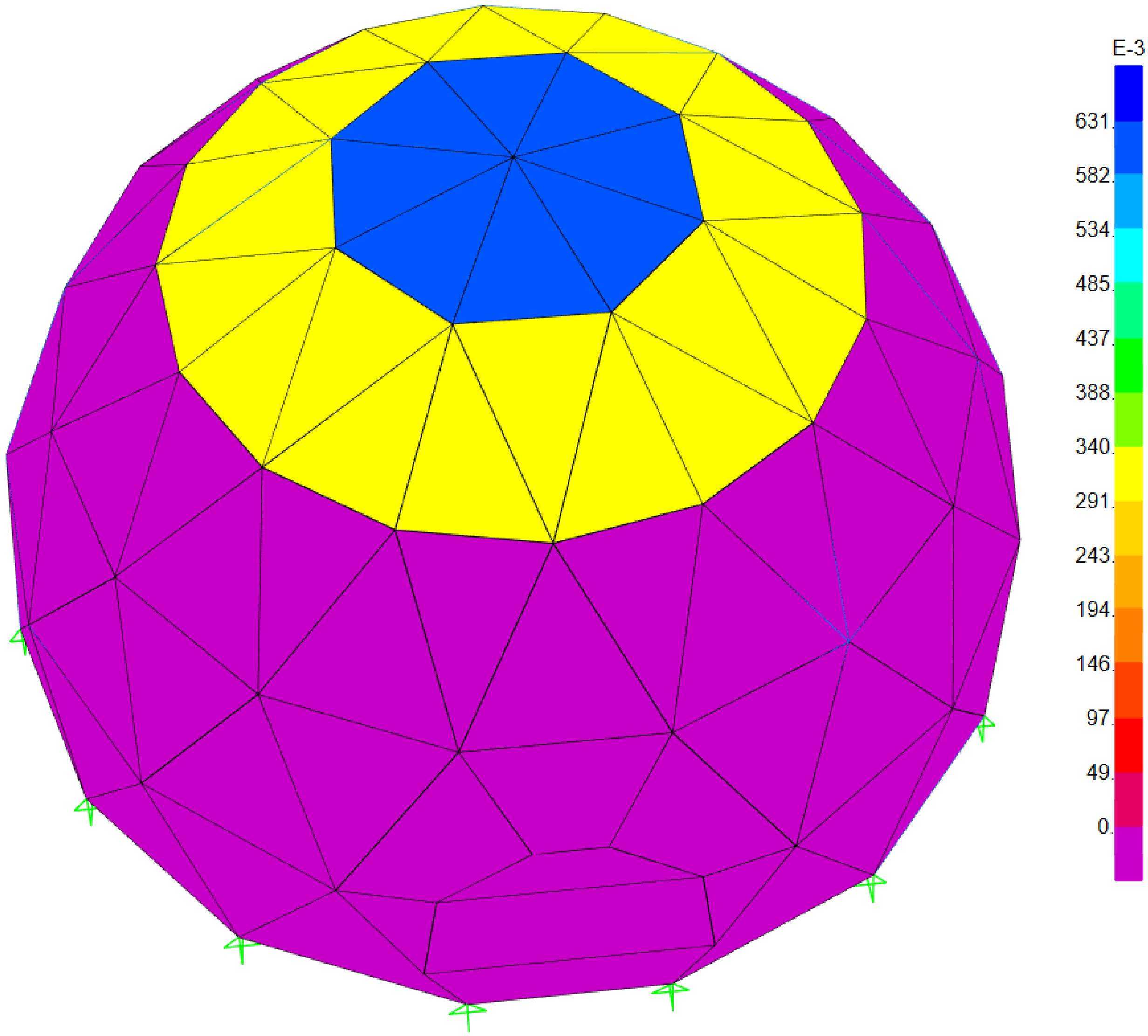
7.2.2 Snow Load (Case2)



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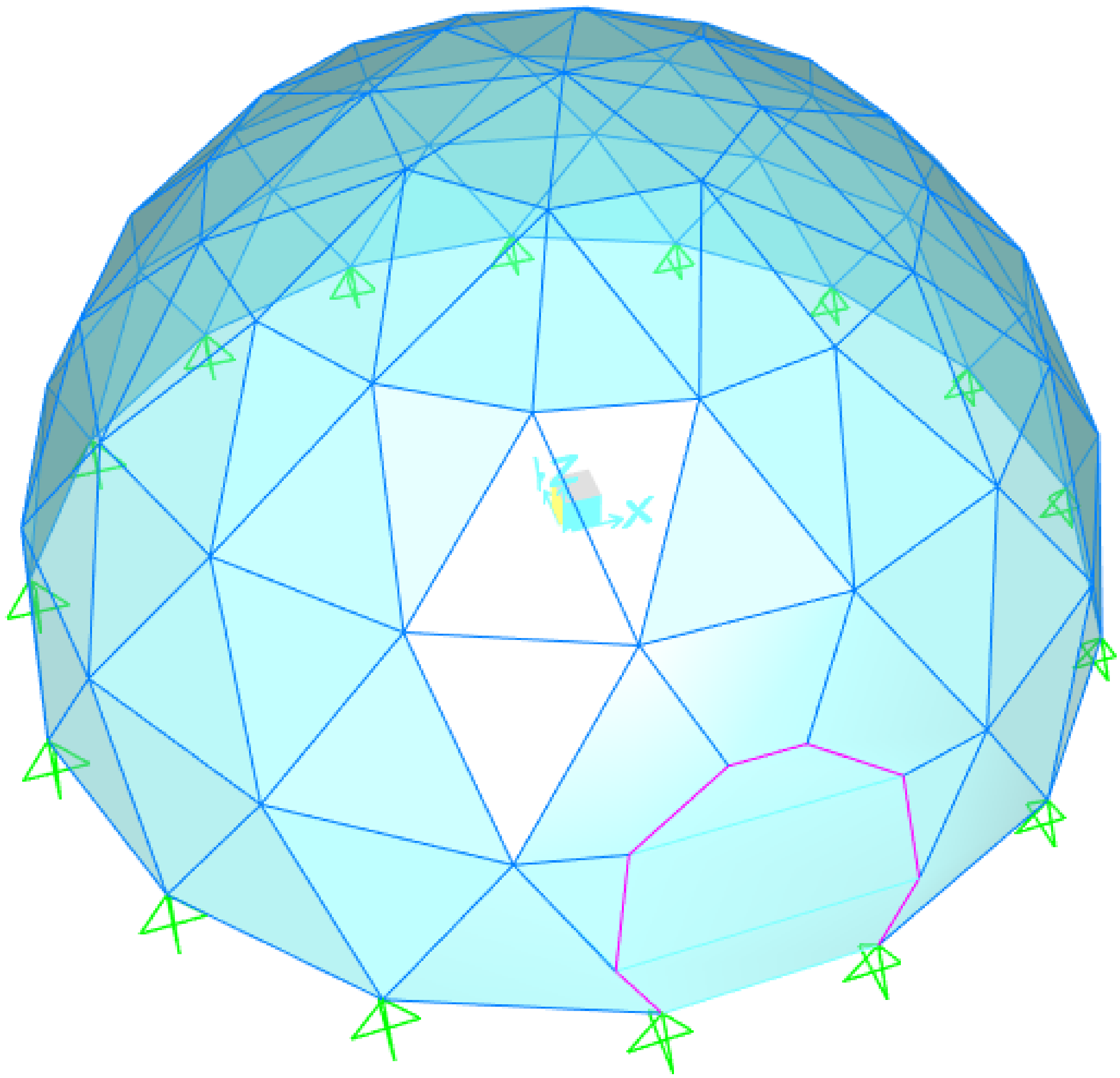
8 Analysis

8.1 3D model



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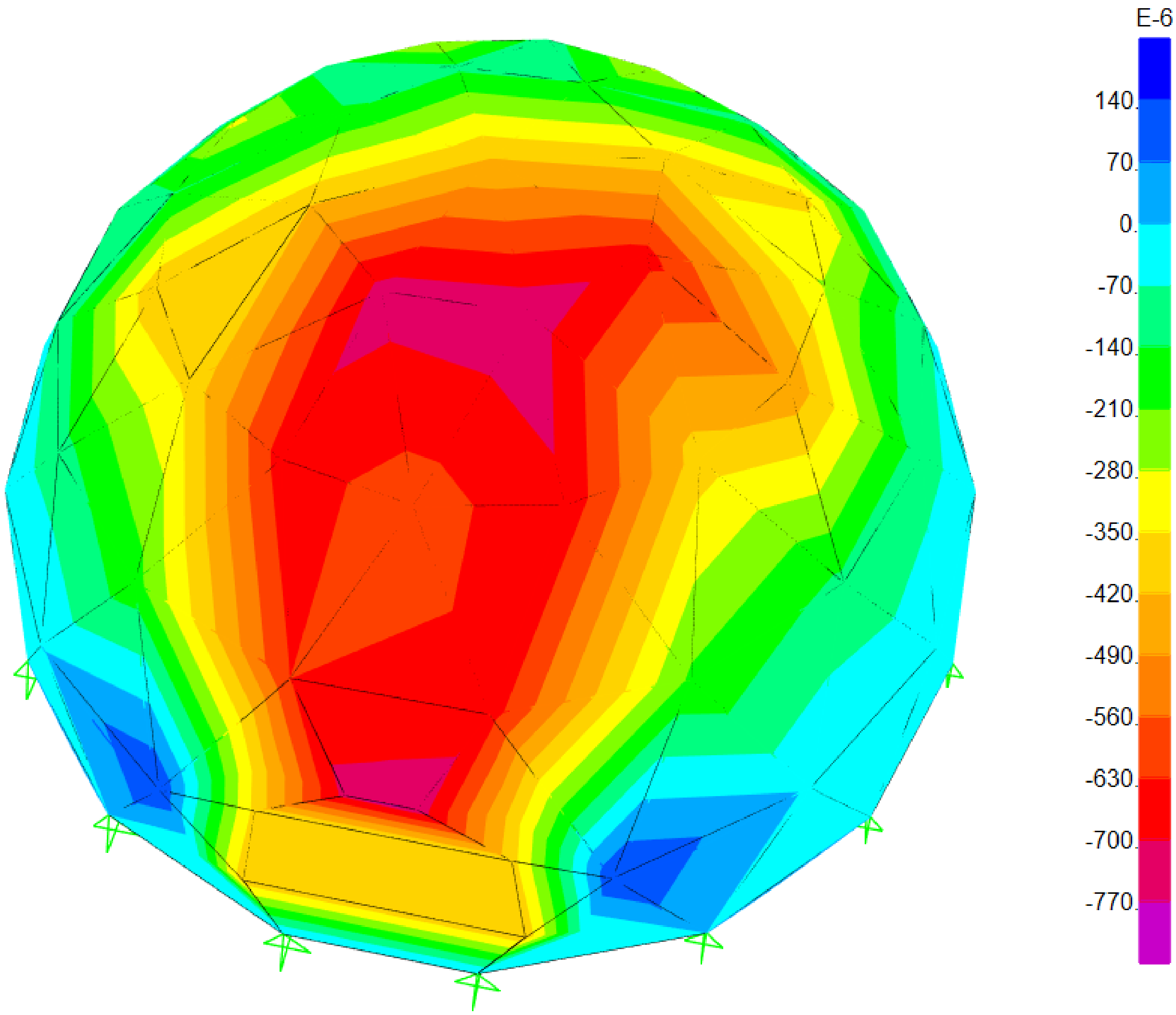
8.2 Results

8.2.1 Maximum vertical deflection (serviceability)



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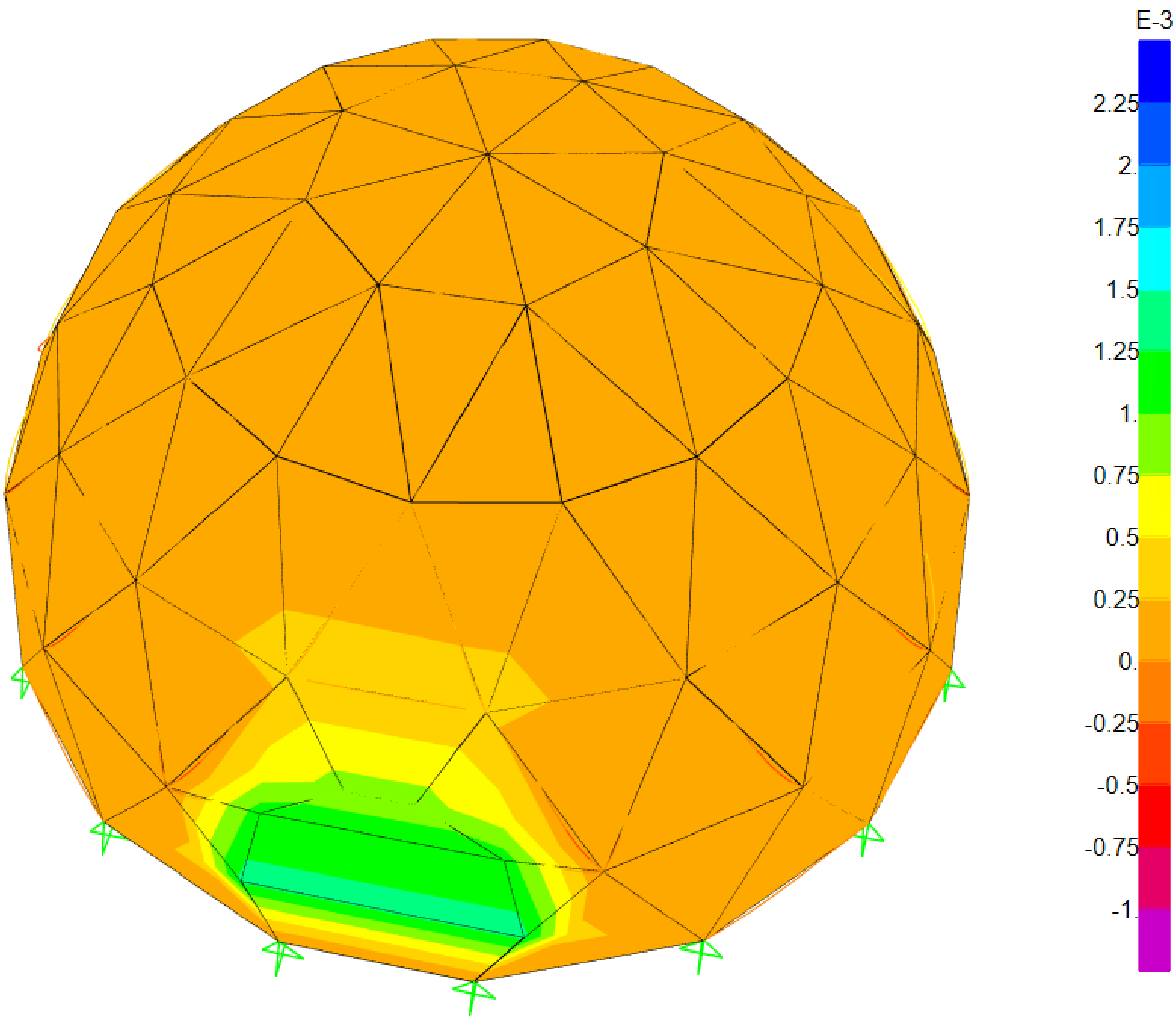
8.2.2 Maximum horizontal deflection (serviceability)



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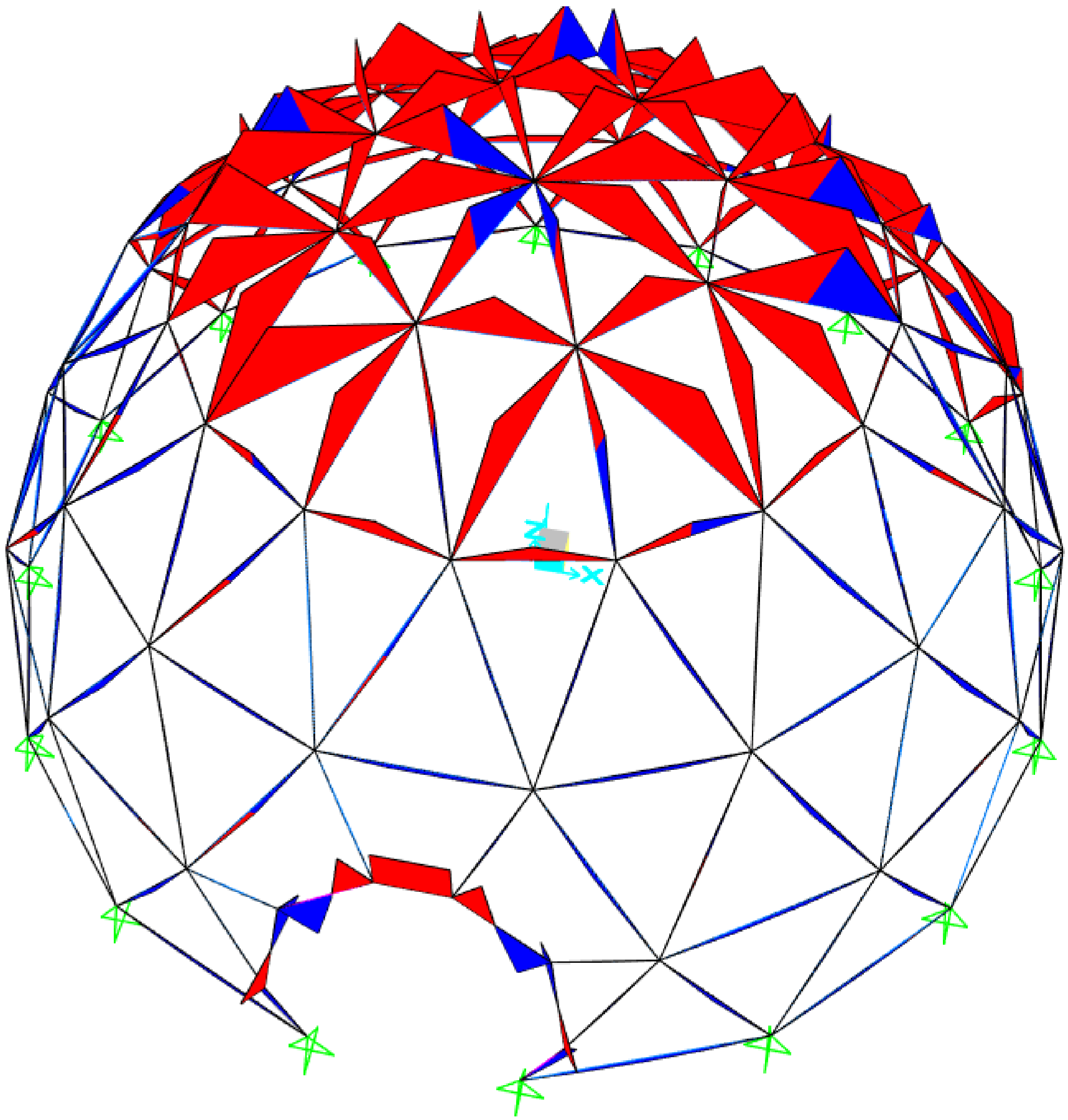
8.2.3 Maximum Bending Moment in Major Axis



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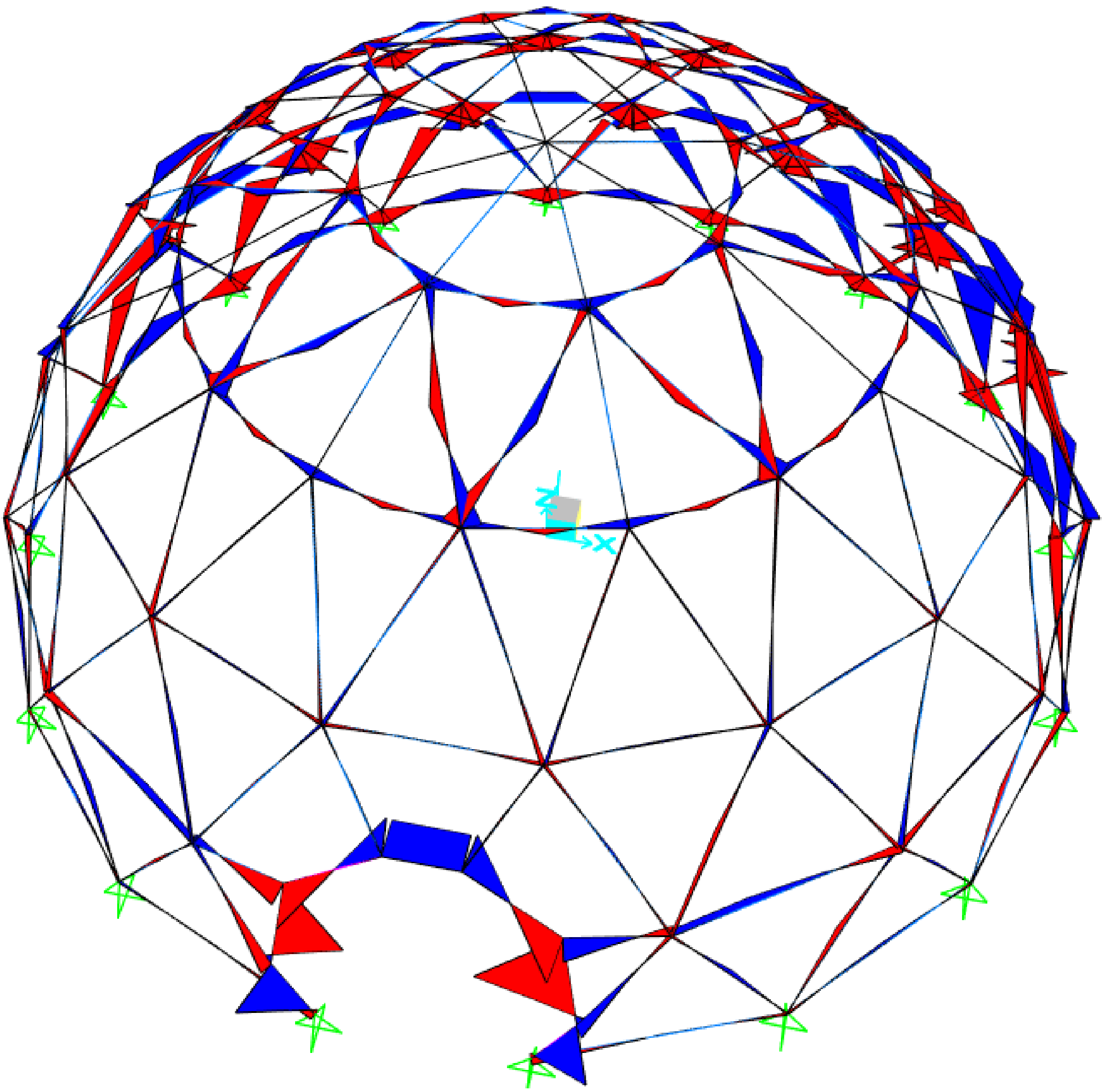
8.2.4 Maximum Bending Moment in Minor Axis



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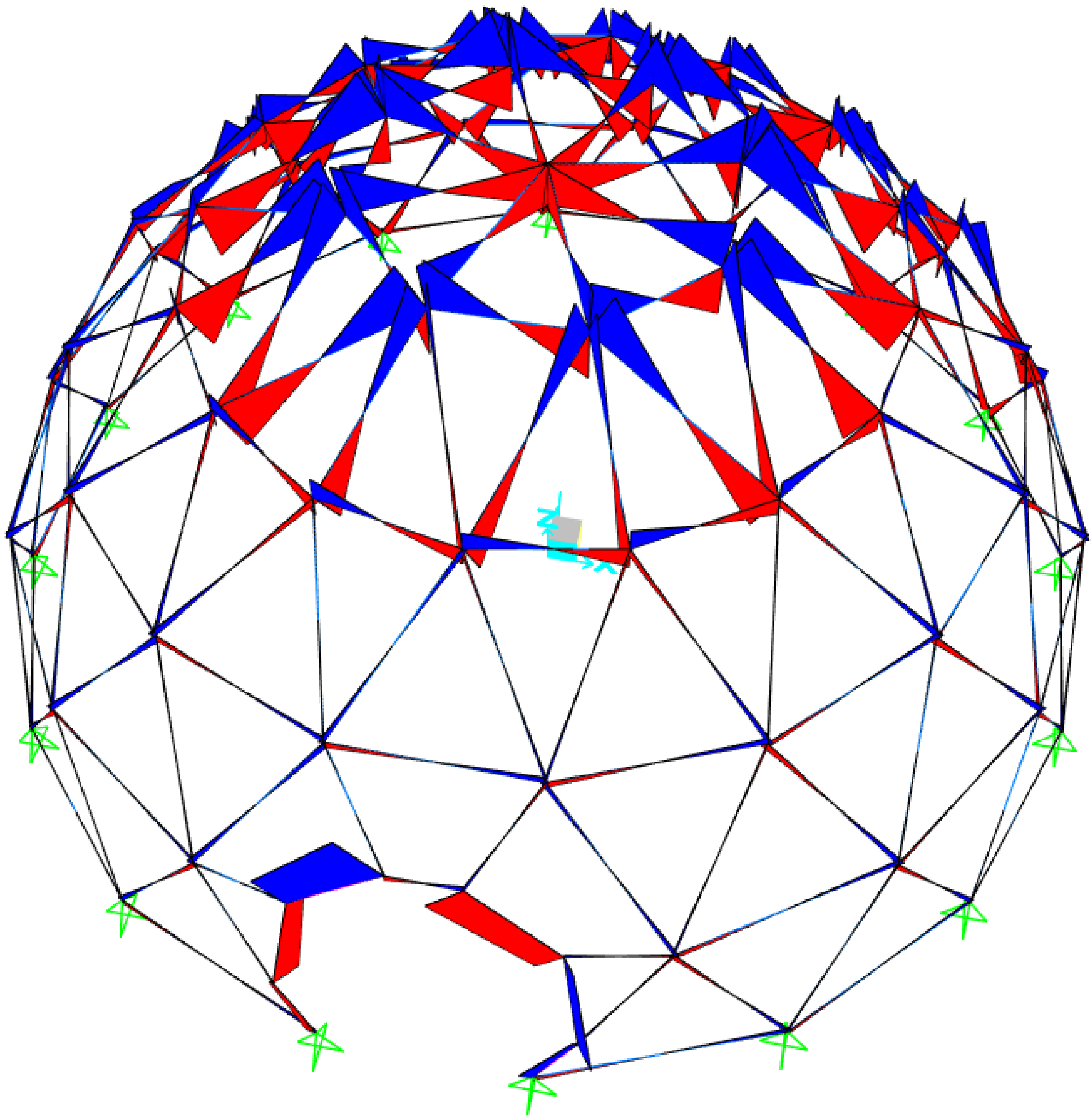
8.2.5 Maximum Shear



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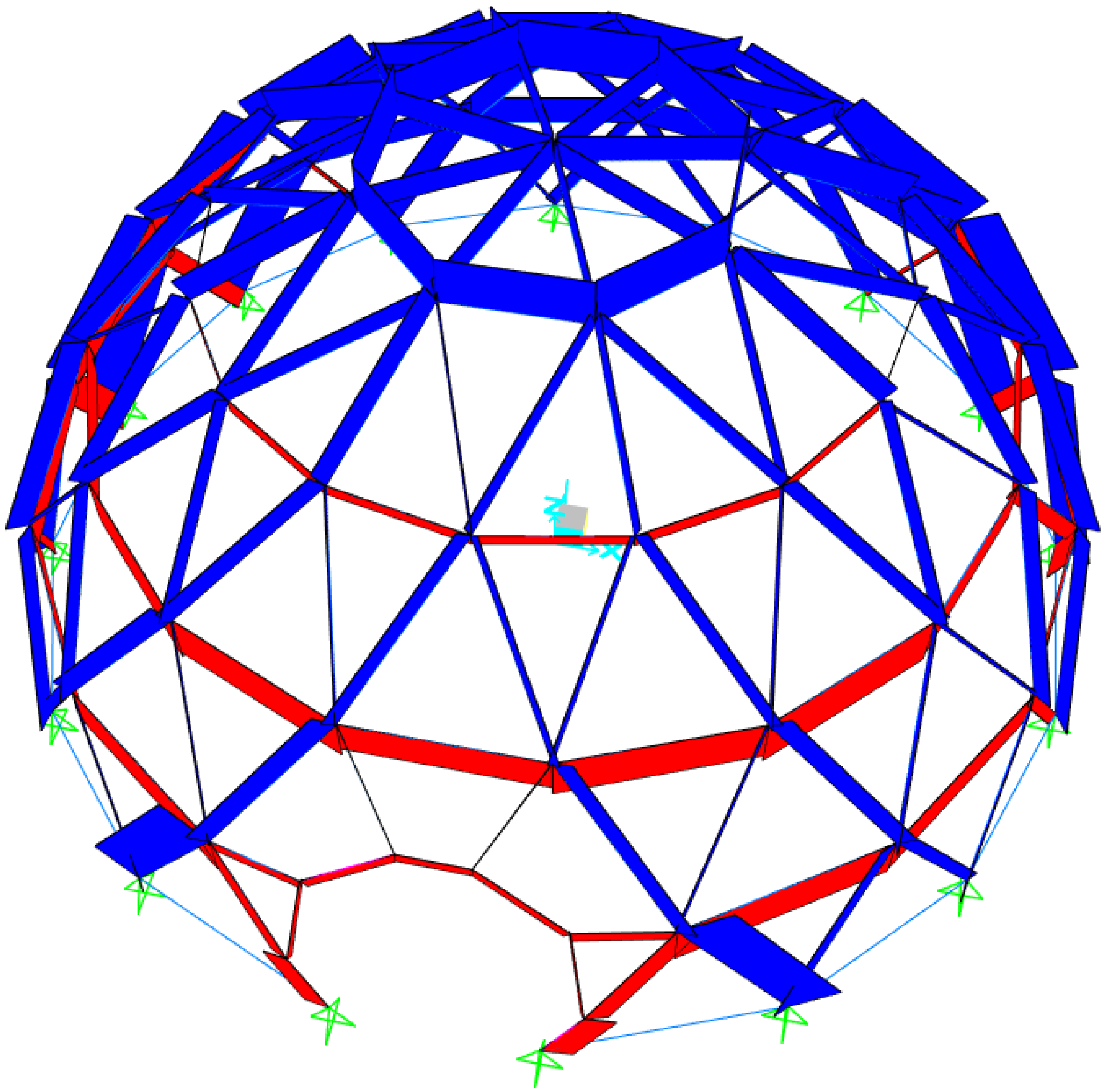
8.2.6 Maximum Axial Force



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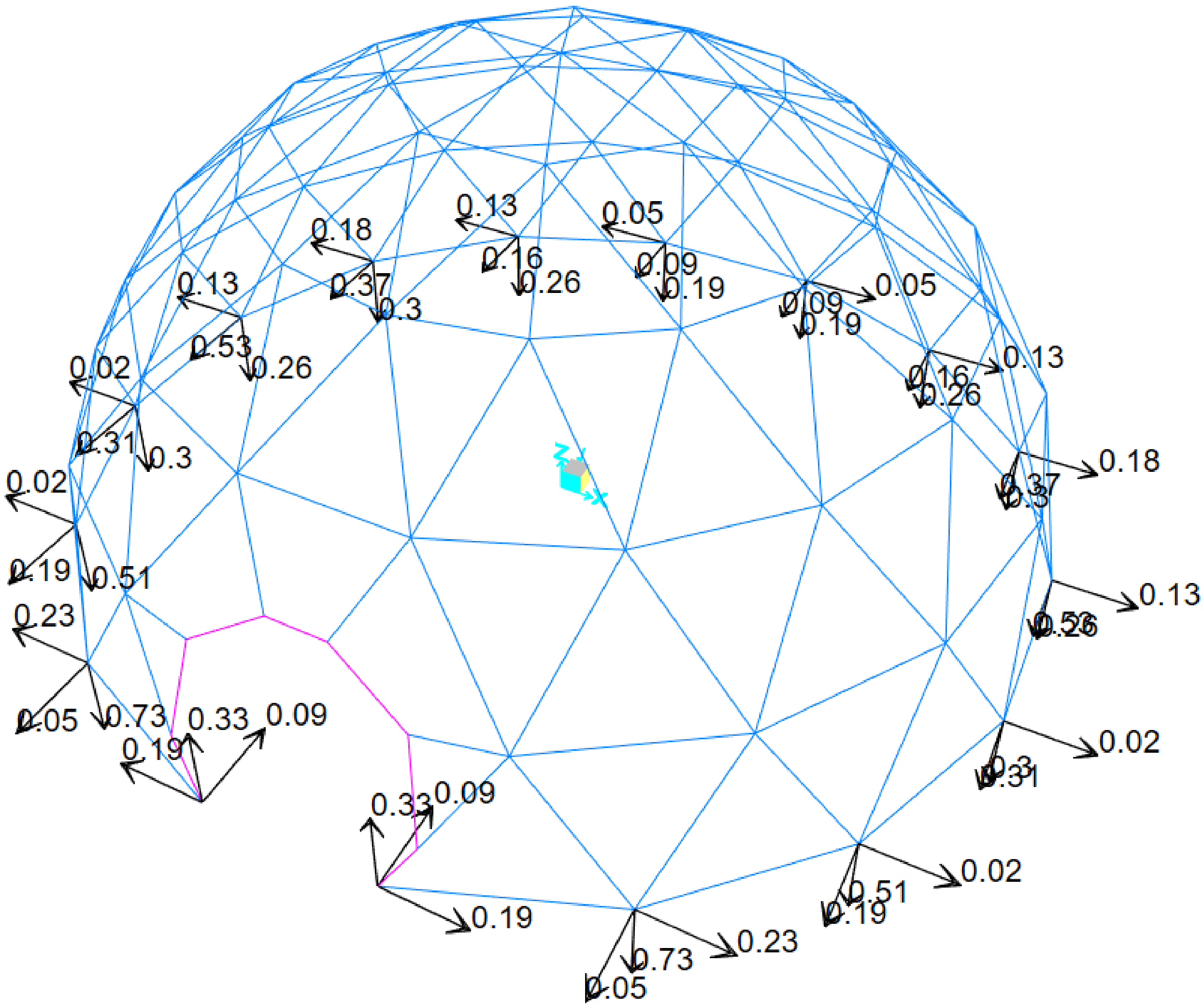
8.2.7 Maximum Reactions



Max. F_x = 0.6 kN
Max. F_y = 0.64 kN
Max. $F_{Z,\ Bearing}$ = 1.35 kN
Max. $F_{Z,\ uplift}$ = 0.8 kN

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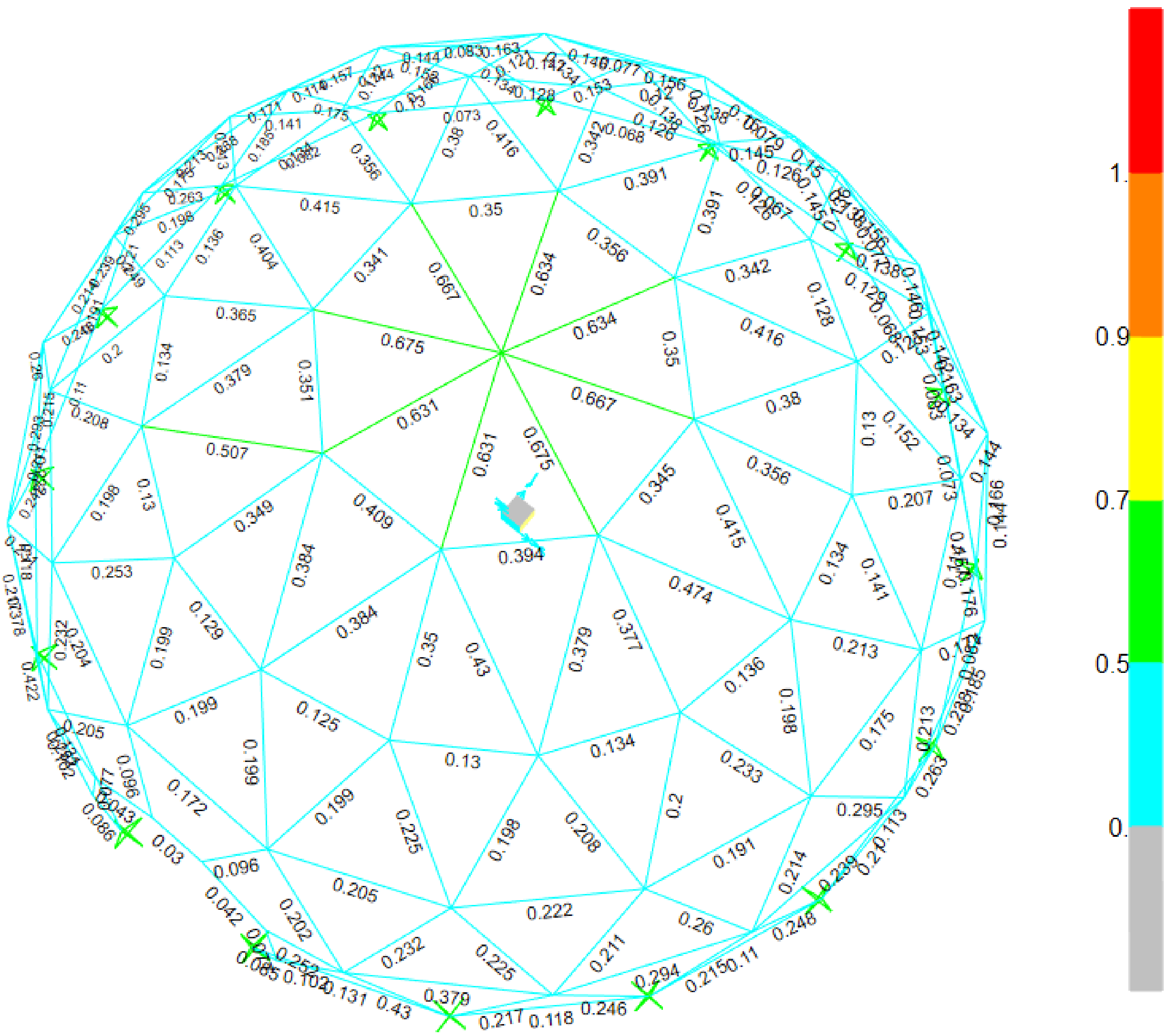
9 Steel Member Design

All members pass for the defined design wind actions. Refer to Appendix 'B' for section capacities and factor of safeties.



All members Pass

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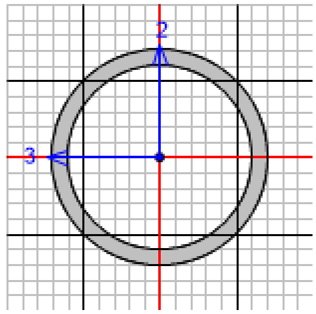
9.1 20x1.5 CHS

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AS 4100-1998 STEEL SECTION CHECK    (Summary for Combo and Station)
Units  :  KN, m, C

Frame :  209       X Mid:  0.571     Combo:  COMB5           Design Type:  Brace
Length:  1.251     Y Mid:  -0.236    Shape:  20x1.5CHS       Frame Type:   Braced Frame
Loc   :  0.626     Z Mid:  3.902     Class:  Compact         Princpl Rot: 0. degrees

PhiB=0.9           PhiC=0.9          PhiTY=0.9          PhiTF=0.9          PhiS=0.9

A=8.718E-05        I33=3.754E-09     r33=0.007          Z33=3.754E-07      Av3=4.378E-05
J=7.508E-09        I22=3.754E-09     r22=0.007          Z22=3.754E-07      Av2=4.378E-05
E=206000000.       Fy=235000.        Ry=1.106           S33=5.145E-07      Iw=0.
RLLF=1.            Fu=370000.        SteelType=HR       S22=5.145E-07


STRESS CHECK FORCES & MOMENTS  (Combo COMB5)
    Location               N*         M33*        M22*         V2*         V3*          T*
    0.626              -0.812        0.049   1.307E-06       0.009   1.880E-05   7.952E-07

PMM DEMAND/CAPACITY RATIO   (8.4.4.1)
    D/C Ratio:    0.675 = 0.675                                    <   0.95          OK
                        = M33*/(phi*Mo33)

BASIC FACTORS
    Buckling Mode    K Factor    L Factor        KL/r
    Major Flexure          1.          1.     190.705
    Minor Flexure          1.          1.     190.705
    Major Braced           1.          1.     190.705
    Minor Braced           1.          1.     190.705
    LTB                   1.4          1.     266.987

AXIAL FORCE & BIAXIAL MOMENT DESIGN   (8.4.4.1)
    Factor                  L   Braced ke     Sway ke     Delta_b     Delta_s     Cm Betam
    Major Bending          1.          1.          1.         1.2          1.     1.    -1.
    Minor Bending          1.          1.          1.          1.          1.   0.28    0.8

                         Lltb          Kt          Kl          Kr     Alpha_m     Alpha_s
    LTB Factors            1.          1.         1.4          1.       1.388       0.981

                   Steel Type          Kf          Kt     Alpha_a     Alpha_b     Alpha_c
    Axial Factors          HR          1.          1.      10.774         -1.       0.226

                      Element    Lambda_e   Lambda_ep   Lambda_ey   Lambda_ew Compactness
    Bending               Any      12.533         50.        120.   1.000E+14     Compact
    Axial                 Any      12.533                     82.                 Compact
```


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	ZeMajor	ZeMinor	de	Aeff
Effective Pro	5.145E-07	5.145E-07	0.02	8.718E-05

	M*	Ms	Mr	Mi	Nc
Major Moment	0.059	0.121	0.116	0.097	4.635
Minor Moment	1.307E-06	0.121	0.116	0.097	4.635

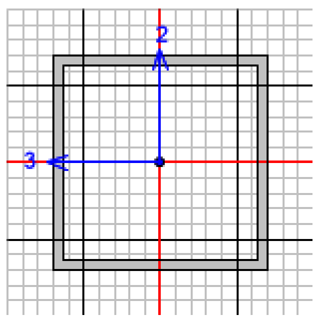
	Mo,cr	Mb	Mo	Mc	Mt
Major Moment	1.216	0.121	0.097	0.097	0.097

	N*	Ns	Nc	Nt	Noz
Axial	-0.812	20.487	4.635	20.487	6907.275

SHEAR CHECK

	V* Force	Vv Capacity	Stress Ratio	Status Check
Major Shear	0.009	6.638	0.001	OK
Minor Shear	1.880E-05	6.638	2.832E-06	OK

9.2 30x2 SHS



AS 4100-1998 STEEL SECTION CHECK (Summary for Combo and Station)
Units : KN, m, C

Frame : 288	X Mid: -0.829	Combo: COMB8	Design Type: Brace
Length: 0.513	Y Mid: -3.876	Shape: 40x40x2	Frame Type: Braced Frame
Loc : 0.513	Z Mid: 0.215	Class: Compact	Princpl Rot: 0. degrees

PhiB=0.9	PhiC=0.9	PhiTY=0.9	PhiTF=0.9	PhiS=0.9

A=3.040E-04	I33=7.337E-08	r33=0.016	Z33=3.668E-06	Av3=1.600E-04
J=1.097E-07	I22=7.337E-08	r22=0.016	Z22=3.668E-06	Av2=1.600E-04
E=206000000.	Fy=235000.	Ry=1.106	S33=4.336E-06	Iw=0.
RLLF=1.	Fu=370000.	SteelType=HR	S22=4.336E-06	

STRESS CHECK FORCES & MOMENTS (Combo COMB8)

Location	N*	M33*	M22*	V2*	V3*	T*
0.513	-0.973	0.078	-0.007	-0.145	-0.014	0.004

PMM DEMAND/CAPACITY RATIO (8.4.4.1)
D/C Ratio: 0.086 = 0.086 < 0.95 OK
= M33*/(phi*Mo33)

BASIC FACTORS

Buckling Mode	K Factor	L Factor	KL/r

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```
    Major Flexure          1.          1.      33.044
    Minor Flexure          1.          1.      33.044
    Major Braced           1.          1.      33.044
    Minor Braced           1.          1.      33.044
    LTB                  1.4           1.      46.261

AXIAL FORCE & BIAXIAL MOMENT DESIGN   (8.4.4.1)
    Factor                  L   Braced ke     Sway ke     Delta_b     Delta_s     Cm Betam
    Major Bending          1.          1.          1.       1.001          1. 0.999-0.997
    Minor Bending          1.          1.          1.       1.002          1.    1.   -1.

                         Lltb          Kt          Kl          Kr     Alpha_m     Alpha_s
    LTB Factors            1.          1.         1.4          1.       1.796       1.027

                   Steel Type          Kf          Kt     Alpha_a     Alpha_b     Alpha_c
    Axial Factors          HR          1.          1.      15.052         -1.       0.988

    Slenderness      Lambda_e   Lambda_ep   Lambda_ey   Lambda_ew Lambda_e/ey Compactness
    Major/Flange       17.452         30.         45.        180.       0.388     Compact
         /Web          17.452         82.        115.        180.       0.152     Compact
    Minor/Flange       17.452         82.        115.        180.       0.152     Compact
         /Web          17.452         30.         45.        180.       0.388     Compact
    Axial/Flange       17.452                     45.                   0.388     Compact
         /Web          17.452                     45.                   0.388     Compact

                      ZeMajor     ZeMinor        b-be        d-de        Aeff
    Effective Pro   4.336E-06   4.336E-06          0.          0.   3.040E-04

                           M*          Ms          Mr          Mi          Nc
    Major Moment        0.078       1.019       1.019       1.003      70.609
    Minor Moment       -0.007       1.019       1.019       1.003      70.609

                        Mo,cr          Mb          Mo          Mc          Mt
    Major Moment       50.112       1.019       1.003       1.003       1.003

                           N*          Ns          Nc          Nt         Noz
    Axial              -0.973       71.44      70.609       71.44   18014.713

SHEAR CHECK
                           V*          Vv      Stress      Status
                        Force    Capacity       Ratio       Check
    Major Shear         0.145      20.304       0.007          OK
    Minor Shear         0.014      20.304       0.001          OK
```


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10 Pegging Design

PRIME CONSULTING ENGINEERS PTY. LTD

Project: **8m Dome**

Jon no. **22-211-2**

Date: **4/05/2022**

Name	*Value*	*Unit*	*Notes*
minimum embedment depth for lateral bearing:			
Max. Horizontal Force	0.64	*kN*	
Max. Vertical Force	0.8	*kN*	
Number of Pegs	1		
Horizontal Load per peg	0.6	*kN*	
Vertical Load per peg	0.8	*kN*	
Sticking out of Ground	0	*m*	
S (bearing capacity)	150	*kPa*	To be confirmed by the Geotechnical engineer
φ	0.02	*m*	
H	213	*mm*	
M	**0.00**	*kNm*	
γ	19	*kN/m³*	To be confirmed by the Geotechnical engineer
min required Embedment:	**213**	*mm*	
F.S	**5.63**		
	OK		
Bending:			
Profile	φ20mm Peg		
Fy	350	*mPa*	
Ze	785.4	*mm3*	
phi	0.9		
phi Ms	**0.25**	*kNm*	
	OK		
Pull out Checking:			
Clay:			
Cu	25	*kPa*	To be confirmed by the Geotechnical engineer

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α (reduction factor)	1	
Provided Embedment	1200	*mm*
L/d	60	
Rs	0.96	
Perimeter	63	*mm*
Total Surface Area	0.075	*m^2*
min required Embedment:	0.53	*m*
F.S	2.26	
	OK	
Coefficient of Friction	0.6	
Equivalent Ballast	**0.11**	*tonne*

Reference: Foundations of Structures - Dunham, Mc Graw-Hill

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11 Summary and Recommendations

- The 8m Dome Structure as specified is capable of withstanding 3s gust wind speed up to **100km/hr** in region A, TC2.
- The dome structure is required to be dismantled for forecast winds in excess of **100km/hr**.
- The dome structure is designed to withstand snow loads of sub-alpine region (Orange, NSW) with maximum ground snow (Sg) 0.9kPa.
- For uplift due to 100km/hr, 1 kN (100kg) holding down weight/per support is required. (16 anchor points in total). Alternatively, pegging system described in Cl. 10 can be used.

Yours faithfully,

Prime Consulting Engineers Pty. Ltd.

Kevin Zia, BEng, Meng, MIEAust, CPENG NER

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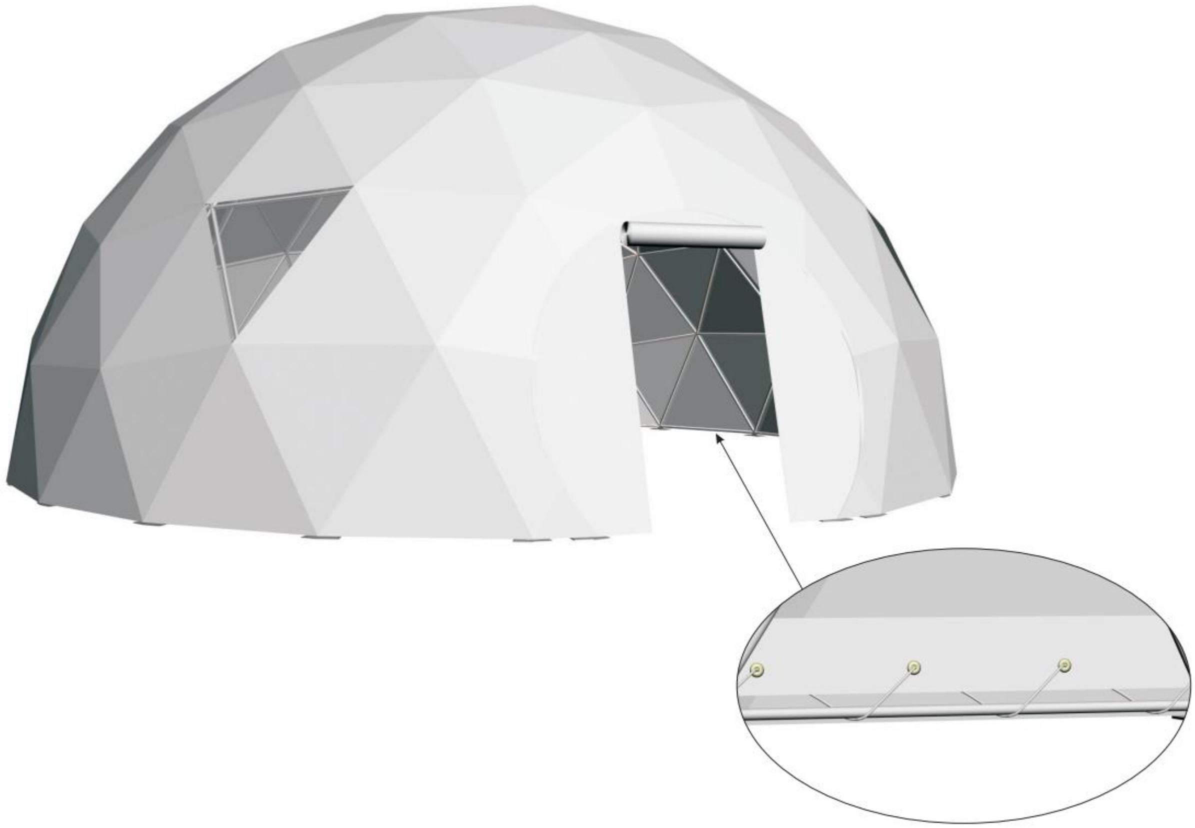
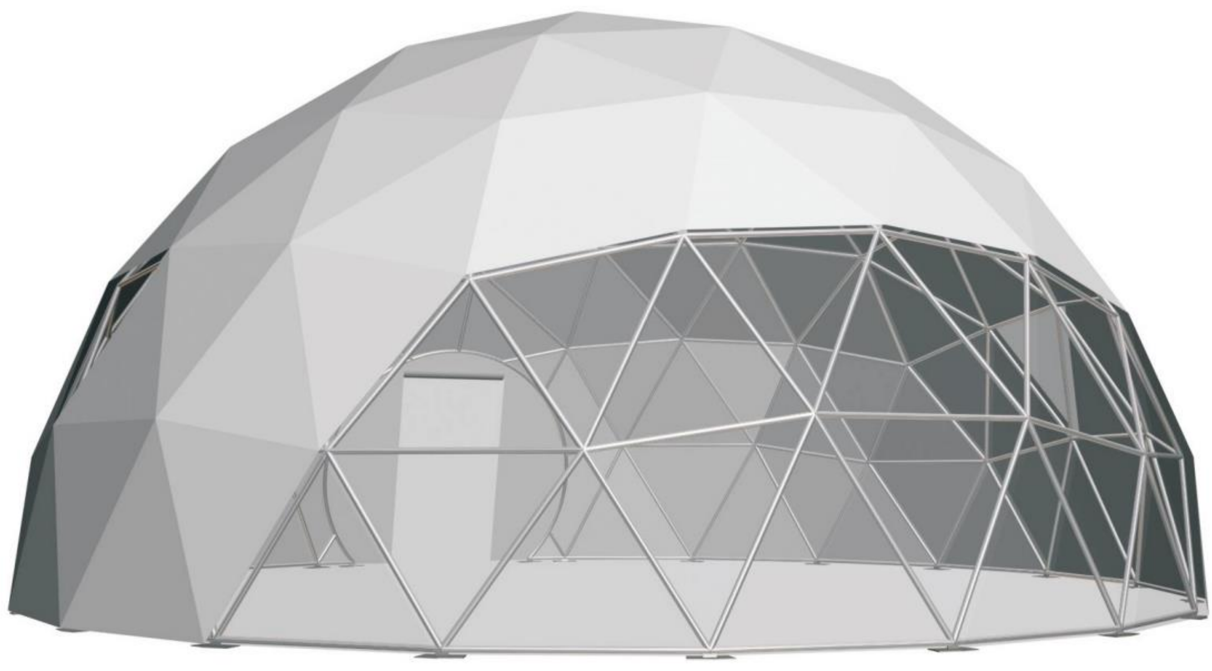
12 Appendix A – Detail Drawings





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EXTREME MARQUEES

WE CAN CUSTOM PRINT ANY COLOUR

DOME RANGE

4m / 5m / 6m / 7m / 8m / 9m / 10m / 11m / 12m

EVENT TENT

ITEM	SPECIFICATION								
Size	4m	5m	6m	7m	8m	9m	10m	11m	12m
Ceiling Height	2.4m	2.5m	3m	3.5m	4m	4.5m	5m	5.5m	6m
Door Size	1200mm W x 1800mm H			1500mm W × 2100mm H			1500mm W x 2400mm H		
Dome Style	Lapela								
Floor Space	12.5 sq.m	17.8 sq.m	28 sq.m	38 sq.m	50 sq.m	63 sq.m	78 sq.m	95 sq.m	113 sq.m
Stand up Capacity	15	24	34	46	60	76	94	114	136
Sit down Capacity	10	16	23	31	40	51	63	76	90
Frame Material	Steel								
Frame Size	20mm diameter x 1.5mm thickness				25mm diameter x 2mm thickness				
Frame Weight	118kg	141kg	186kg	220kg	321kg	397kg	498kg	534kg	736kg
Roof Weight	41kg	64kg	77kg	105kg	137kg	155kg	191kg	231kg	308kg
Bearing Bar Weight	11kg	14kg	17kg	19kg	22kg	25kg	27kg	30kg	33kg
Total Weight	170kg	218kg	279kg	344kg	479kg	576kg	716kg	795kg	1076kg
Installation Time (Hours)	4	4	4	5	5	5	6	7	8
Labourers (QTY)	3	3	4	4	5	5	6	6	6
Equipment Scaffold	2.5m	2.5m	2.5m	2.5m	2.5m	3m	3.5m	4m	N/A
Crane	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	20 Ton Crane
Roof Fabrics	580GSM PVC & Clear PVC								
Roof Type	Solid PVC Roof, Clear PVC Roof or 1/3 Clear 2/3 Solid PVC Roof								
PVC Standard Colours	White, Clear PVC, Red, Yellow, Blue, Green and Black								
Printing	Full Roof Digital Printing								
Package Includes	Steel Pins								
Manufacturer's Warranty	Frame 5 Year Plain PVC 5 Year / Printed PVC 2 Year / Clear PVC 1 Year								

ADDITIONAL INFORMATION

The Extreme Marquee Dome Tent is a visually striking outdoor gazebo with an innovative space age design. The frame is made from strong tubular powder coated steel. Connectors join the tubular bars to form triangular cross sections, which join together to give this structure both its enormous strength and futuristic outer appearance.

We design | We manufacture | We sell | We Deliver Australia wide www.extrememarquees.com.au admin@extrememarquees.com.au 1300 850 832

At Extreme Marquees we design & manufacture all of our products & deliver Australia wide. 17/07/2019

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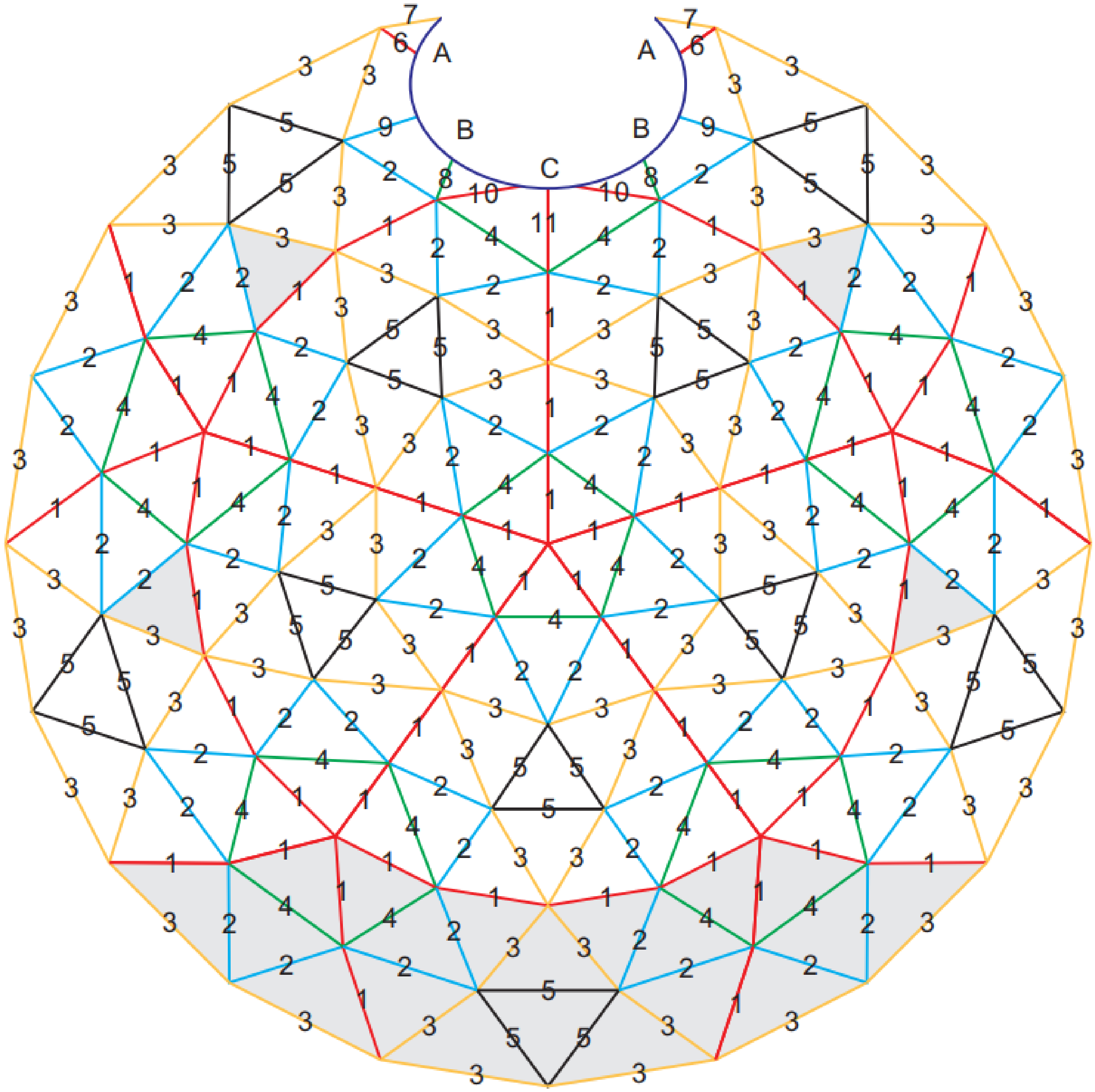

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